BROWARD COUNTY GO SOLAR

BROWARD COUNTY ENVIRONMENTAL PLANNING AND GROWTH MANAGEMENT DEPARTMENT

BROWARD ROOFTOP SOLAR PHOTOVOLTAIC ROOFTOP MOUNTING SYSTEMS

DRAWING INDEX

COV COVER

ARCHITECTURAL

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CARTAYA & ASSOCIATES ARCHITECTS, P.A.

3077 E. COMMERCIAL BLVD. SUITE 201 FT. LAUDERDALE, FL. 33308 954-771-2724 (ARCHTECTURE)

TY LIN INTERNATIONAL, INC.

201 ALHAMBRA CIRCLE, SUITE 900 CORAL GABLE, FL. 33134 305-567-1888 (STRUCTURAL/M.E.P./ENGINEERING)

CARTAYA & ASSOCIATES ARCHITECTS P.A.

3077 E. COMMERCIAL BLVD. SUITE 201
FT. LAUDERDALE, FLORIDA 33308
771-2724 FAX 776-4280

BEU AA C001888

PROJECT: PHOTOVOLTAIC SYSTEM PROTOTYPE DESIGN
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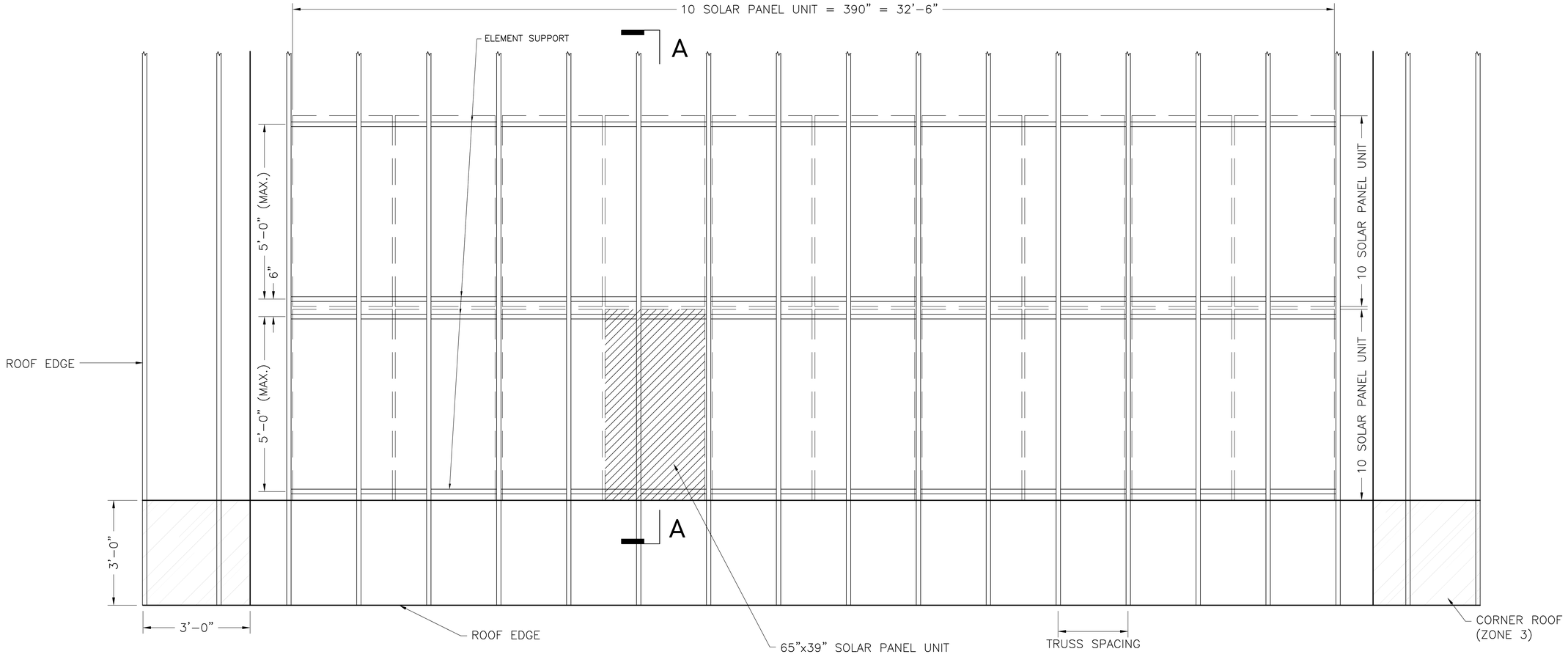
TITLE: COVER SHEET

JOB NO: 1236

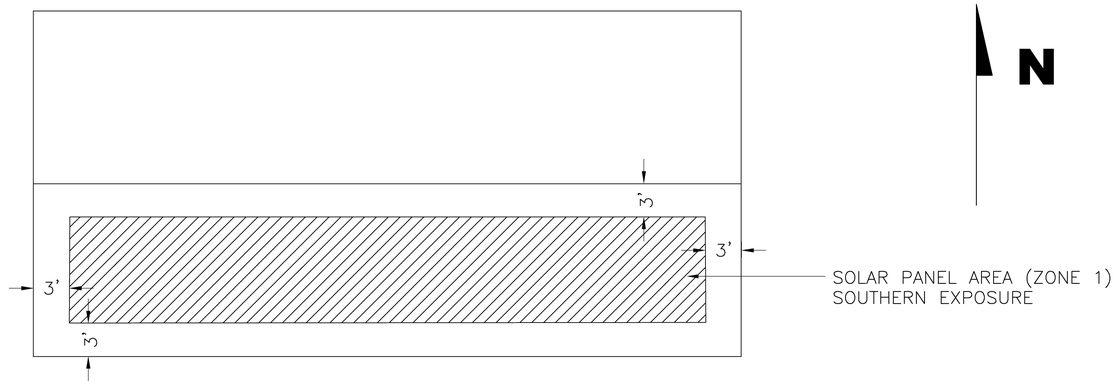
SHEET: COV
1 OF 1



PLAN VIEW

NTS

LAYOUT FOR 65"x39" SOLAR PANEL UNIT ON ROOF ZONE 1 PER FBC 2010
(LOCATION SHALL NOT BE WITHIN ZONES 2 AND 3 AS SHOWN)



ROOF PLAN

NTS

CARTAYA & ASSOCIATES
ARCHITECTS P.A.
3077 E. COMMERCIAL BLVD. SUITE 201
FT. LAUDERDALE, FLORIDA 33308
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BEU AA C001688

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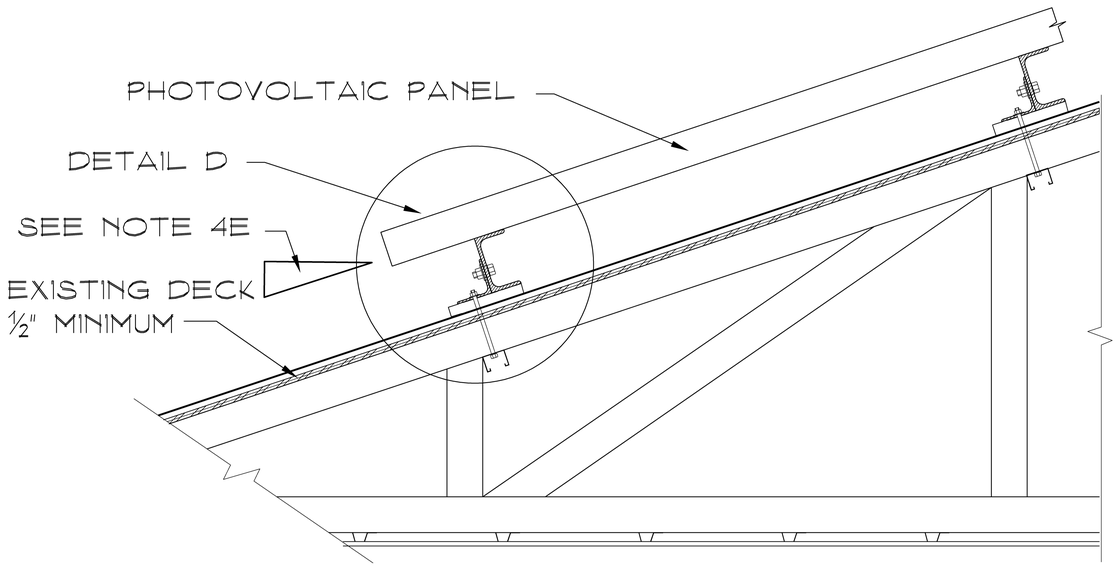
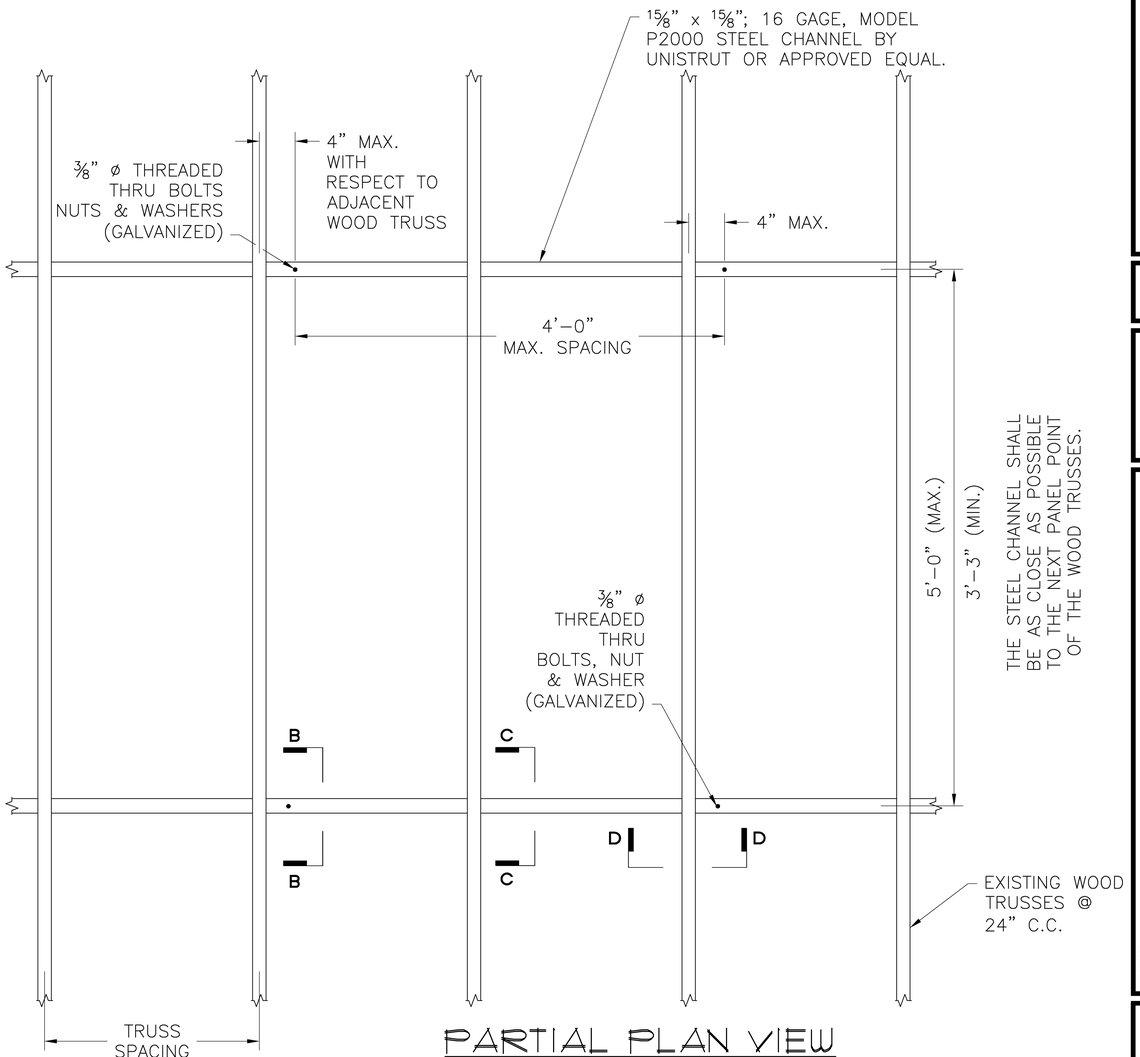
SHEET
A-1
1 OF 5



A / A-1 SECTION

SCALE: 1" = 1'-0"



PARTIAL PLAN VIEW
SOLAR PANEL ATTACHMENT

GENERAL NOTES:

1. DESIGN LOAD CRITERIA

WIND LOAD

a. FLORIDA BUILDING CODE 2010
b. SCE 7-10, HURRICANE PRONE REGION
c. BASIC SPEED - 170 mph
d. EXPOSURE CATEGORY : C
e. RISK CATEGORY : II

2. STRUCTURAL STEEL AND BOLTS SHALL CONFORM TO ASTM A36

3. ALL METALLIC AND STEEL ELEMENTS THAT INVOLVED IN THE FIXING PANEL SOLAR SYSTEM MUST BE HOT-DIP GALVANIZED.

4. CONSIDERATION:
a. THESE NOTES SET OUT THE PARAMETERS UNDER WHICH THE SOLAR PANELS MUST BE INSTALLED.
b. BROWARD COUNTY
c. RESIDENTIAL
d. 30' MAXIMUM HEIGHTS IN RESIDENTIAL ROOF
e. THE SLOPE OF THE ROOF MAY BE FROM 0° TO 45° MAX
f. SOLAR PANEL MUST WITHSTAND WIND LOADS. THE FOLLOWING MAX. LOADS:
   - DOWNWARD DIRECTION: + 70 PSF
   - UPWARD DIRECTION: -79 PSF

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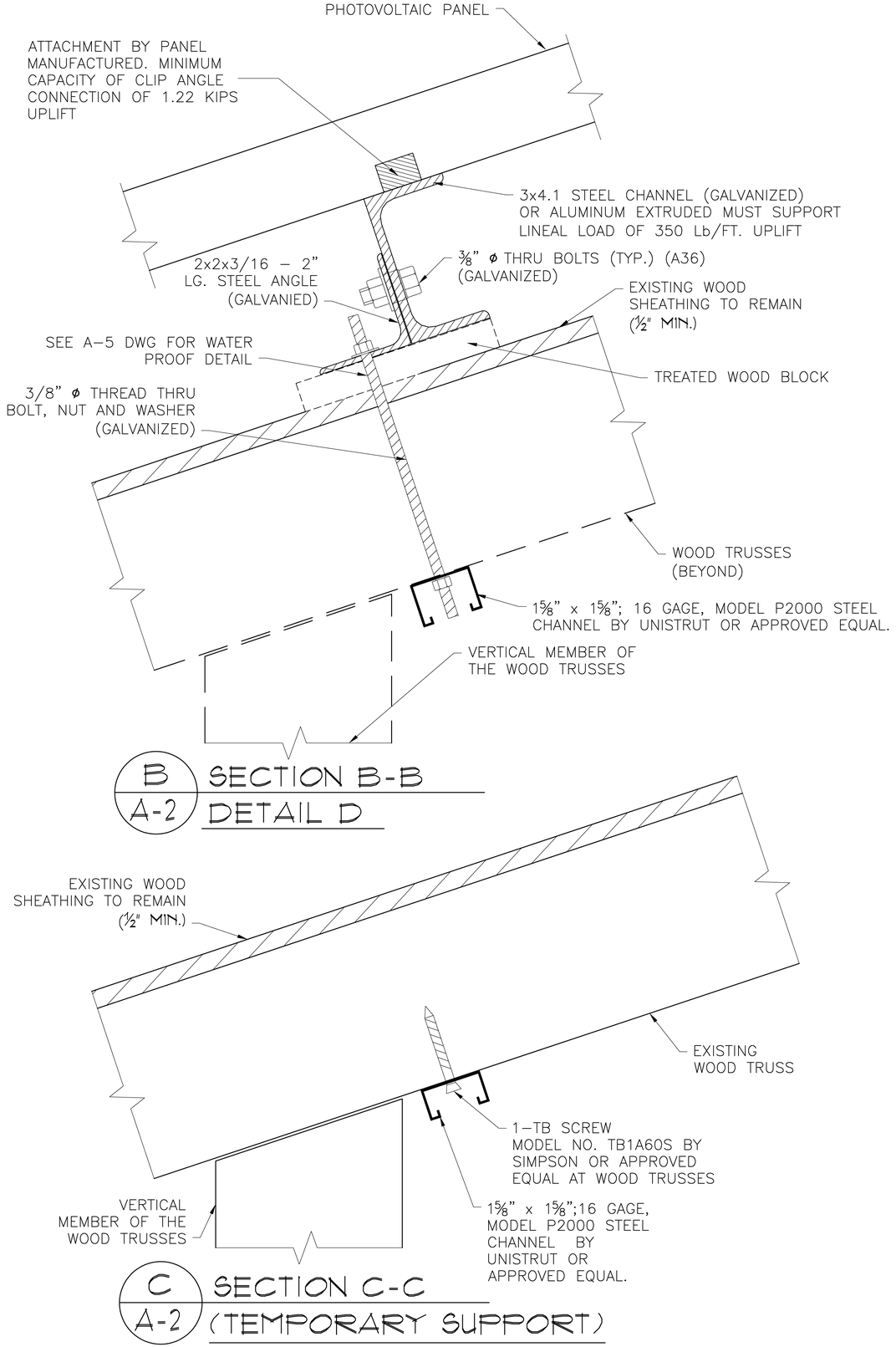
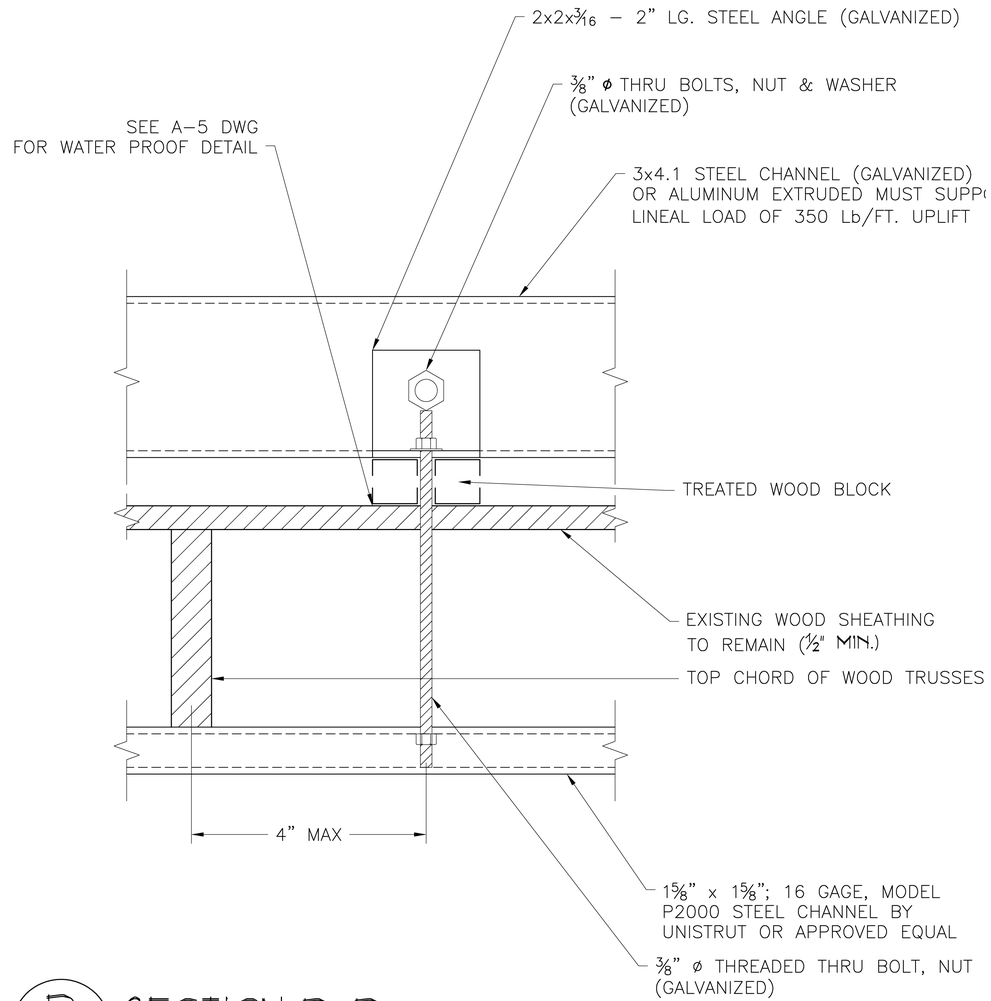
2 OF 5



B / A-2 SECTION B-B
DETAIL D

C / A-2 SECTION C-C
(TEMPORARY SUPPORT)



D / A-2 SECTION D-D

CARTAYA & ASSOCIATES ARCHITECTS P.A.
3077 E. COMMERCIAL BLVD. SUITE 201
FT. LAUDERDALE, FLORIDA 33308
771-2724 FAX 776-4281
BEU AA C001888

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SHEET
A-3
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WIND LOAD PRESSURE

Using Rooftop Structures and Equipments (29.5.1 Sheet 308) Broward County

Residential: II risk category. (Table 1.5.1/ Sheet 2)

V= 170 Mph

K_d= 0.85 (Table 26.6-1/ Sheet 250)
K_{zt}= 1.0

Z= 15' → K_z = 0.85 (Table 29.3-1/ Sheet 310)
Z= 20' → K_z = 0.90
Z= 25' → K_z = 0.94
Z= 30' → K_z = 0.98

$Q = 0.00256\ K_z \times K_{zt} \times K_d \times V^2$

$q_{15'}$ = 53.4 lb/ft^2
$q_{20'}$ = 56.6 lb/ft^2
$q_{25'}$ = 59.1 lb/ft^2
$q_{30'}$ = 61.6 lb/ft^2

Note:

1- We use zone 2 for the calculation of wind load, assuming that the panels may be installed in zone 1 and 2.
2- We consider that the panels are part of the roof and GC_{pi} = ± 0.18 (Worst Condition)

Θ≤7° (Gable Roof) Fig 30.A-2A/ Sheet 336
GC_{p12S} = +0.3
GC_{p2} = -1.8 **(This coefficient value is the most critical for the upward load.)**

7°<Θ≤27° (Gable/ Hip Roof) Fig 30.4-2B/ Sheet 337)
GC_{p12S} = +0.5
GC_{p2} = -1.7

27°<Θ≤45° (Gables Roof) Fig 30.4-2C/ Sheet 338
GC_{p12S} = +0.9 **(This coefficient value is the most critical for the downward load.)**
GC_{p2} = -1.2

$P = q_h\ [(GC_p) - (GC_{pi})]$

27°<Θ≤45° h = 30'
p_{12S} = 61.6[0.9-(-0.18)] = +66.5 lb/ft^2 **(Downward Pressure)**

Θ≤7° h = 30'
p_2 = 61.6[-1.8-(+0.180] = 122 lb/ft^2 **(Upward Pressure)**

27°<Θ≤45° (Monoslope roof) Fig 30.4-5A/ Sheet 341
GC_{p12S} = +0.3
GC_{p2} = -1.7

CONNECTION DESIGN

Panel Point Spacing 5'-0" Max

Uplift @ Connect

$$F = 122\frac{lb}{ft^2} \times \frac{5"}{2} \times 4' = 1220\ lb/Connection/Upward$$

4': Max Spacing Anchor

Steel Channel Design

$$M = \frac{F \times a \times b}{l} = \frac{1.22 \times 0.33 \times 1.66'}{2'} = 0.334\ k.ft$$

$$S_{Req} = \frac{M}{0.6F_y} = \frac{0.334 \times 1.2"}{0.6 \times 36} = 0.016n^3 < 0.072\ n^3 \rightarrow$$ Steel Channel $1\ ^5/_8" \times\ ^7/_8" \times$ 12gauge S=0.072n^3 I = 0.037n^4

Model No. P3300

Steel Track Design

$$W = \frac{122lb}{ft^2} \times 5'/_2 = 305\ lb/ft$$

$$\Delta = \frac{L}{360} = \frac{4' \times 12"}{360} 0.1333"$$

$$I = \frac{5 \times wl^4 \times 1728}{384 \times 29000 \times \Delta} = \frac{5 \times 0.305 \times 4^4 \times 1728}{384 \times 29000 \times 0.1333} = I = 0.46n^4$$

Use: [3x4.1 Steel Channel (I= 1.66n^4, S= 1.10 n^3)

Steel Bolt Anchor Design

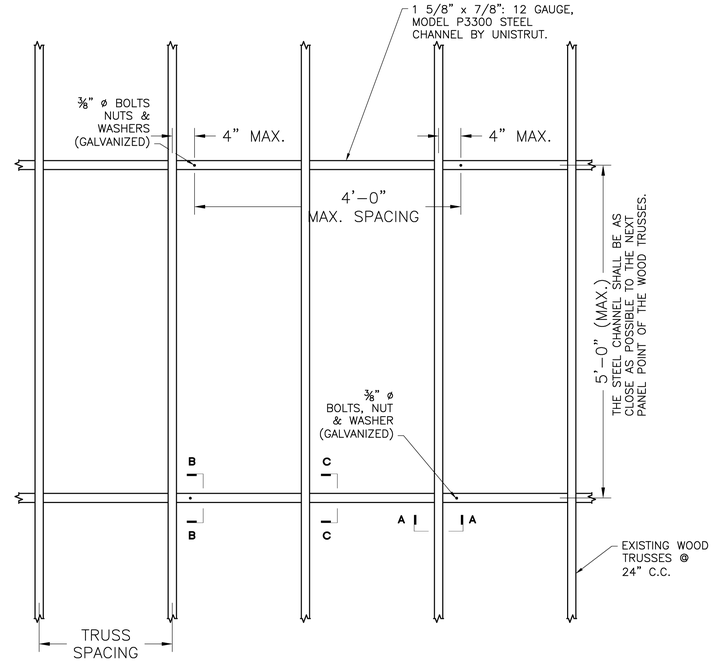
F= 1.22k Tension

3/8" Φ area = πR^2= 0.1104n^2

For A36 Bolt → F_v= 58 k/n^2

F_t= 0.33F_v= 19.14 k/n^2

$F_{Allowed}$= A x F_t= 0.1104 x 19.14 k/n^2 = 2.1^k > 1.22^k (Tension) OK



SOLAR PANEL ATTACHMENT
PLAN VIEW

CARTAYA & ASSOCIATES
ARCHITECTS P.A.
3077 E. COMMERCIAL BLVD. SUITE 201
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771-2724 FAX 776-4280
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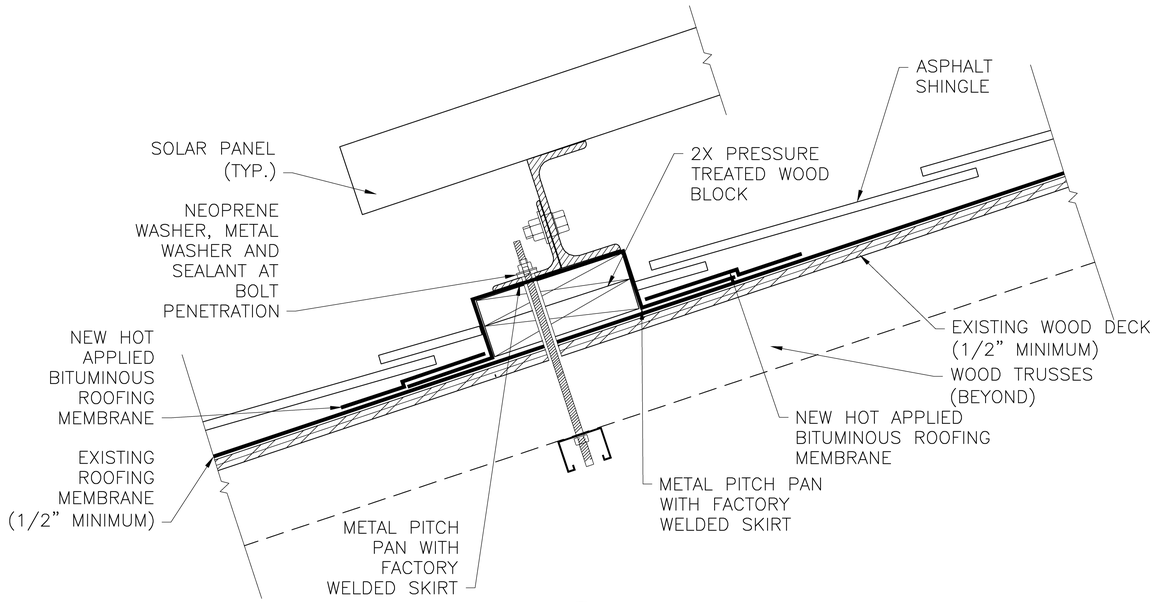
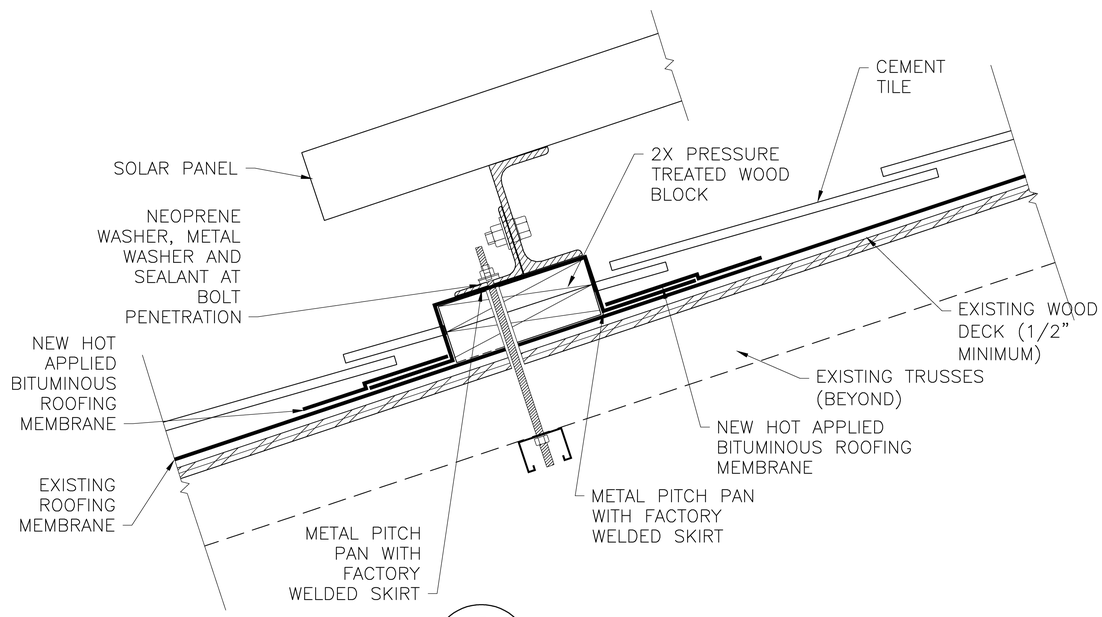
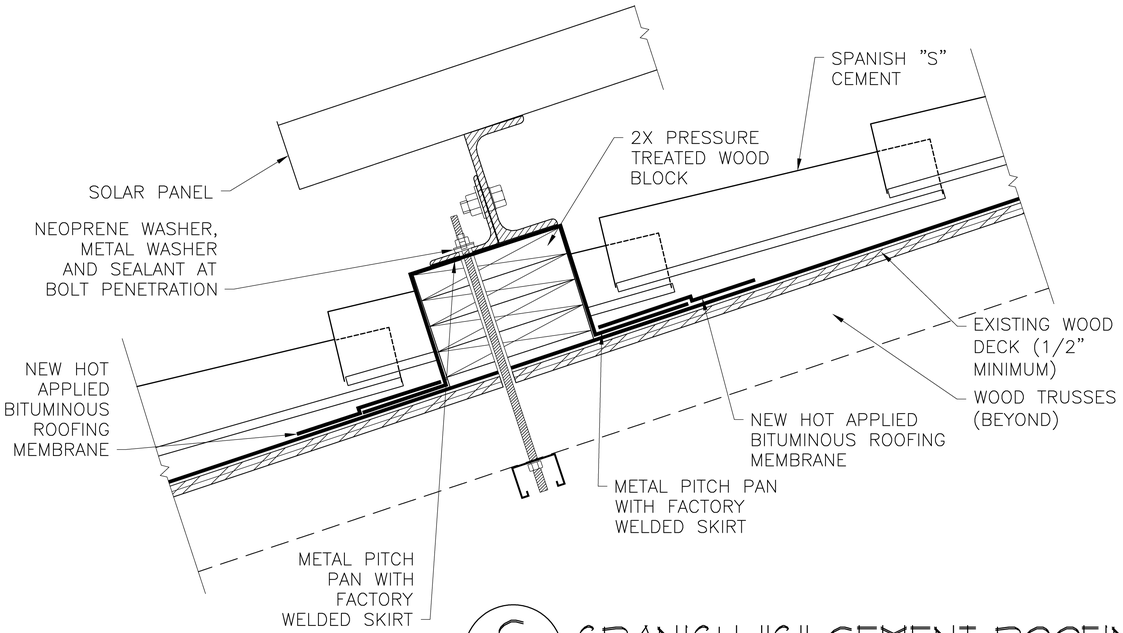


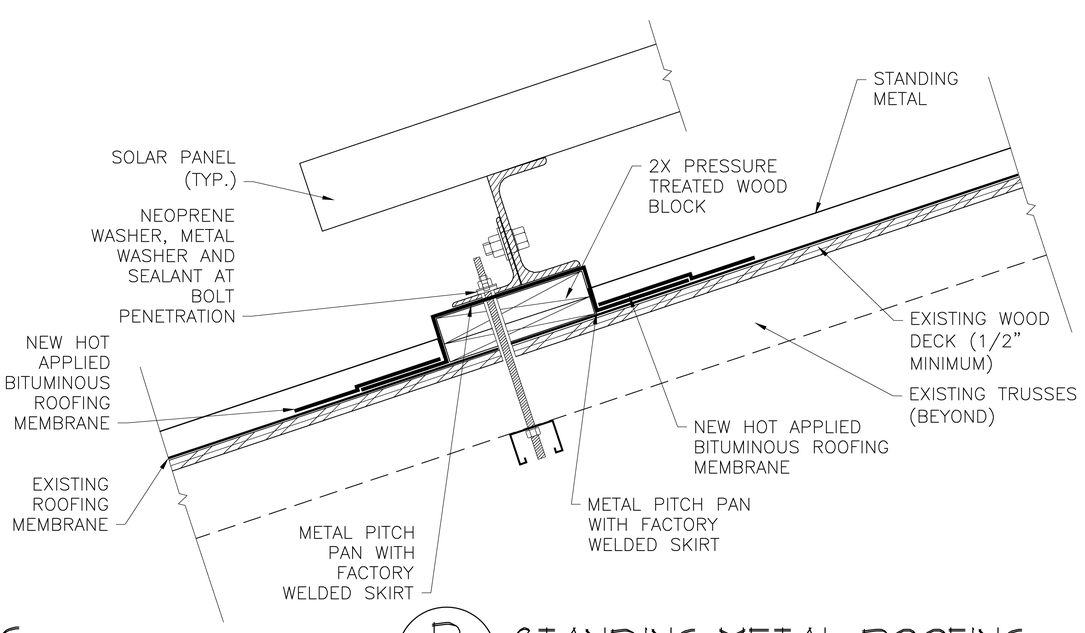
A / A-5 ASPHALT SHINGLE ROOFING WATERPROOFING DETAIL



B / A-5 CEMENT TILE ROOFING WATERPROOFING DETAIL



C / A-5 SPANISH "S" CEMENT ROOFING WATERPROOFING DETAIL



D / A-5 STANDING METAL ROOFING WATERPROOFING DETAIL

CARTAYA & ASSOCIATES
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